

Using a Flow Dynamics 2D Model to Develop Street Flooding Solutions in a Highly Transited Urban Area

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JANUARY 27 – 30
Boston Marriott Copley Place
Boston, Massachusetts

Outline

- Study Motivation
- Description of the Study Area
- Data Collection
- Design Criteria
- Model Setup
- Simulation Results
 - Existing and Proposed Conditions
- Conclusions

Study Motivation

- Recurrent street flooding in a highly transited area



Need to evaluate flooding solution measures

- Use of 2D Overland Model over other approaches:
 - Traditional 1D Catchment Modeling not suited for analysis
 - – Spread Analyses
 - Each CB requires calculation of its own tributary area => time consuming
 - Not direct visualization of flooding extents

Study Area



- LU: Residential/Commercial Area
- Pipe has leftover capacity, thus flooding induced by limited inlet capacity
- Low laying Area
- Low Curbs

Data Collection

- CB Field investigations:
 - Single & Double Grate
 - 10" Lead Pipes



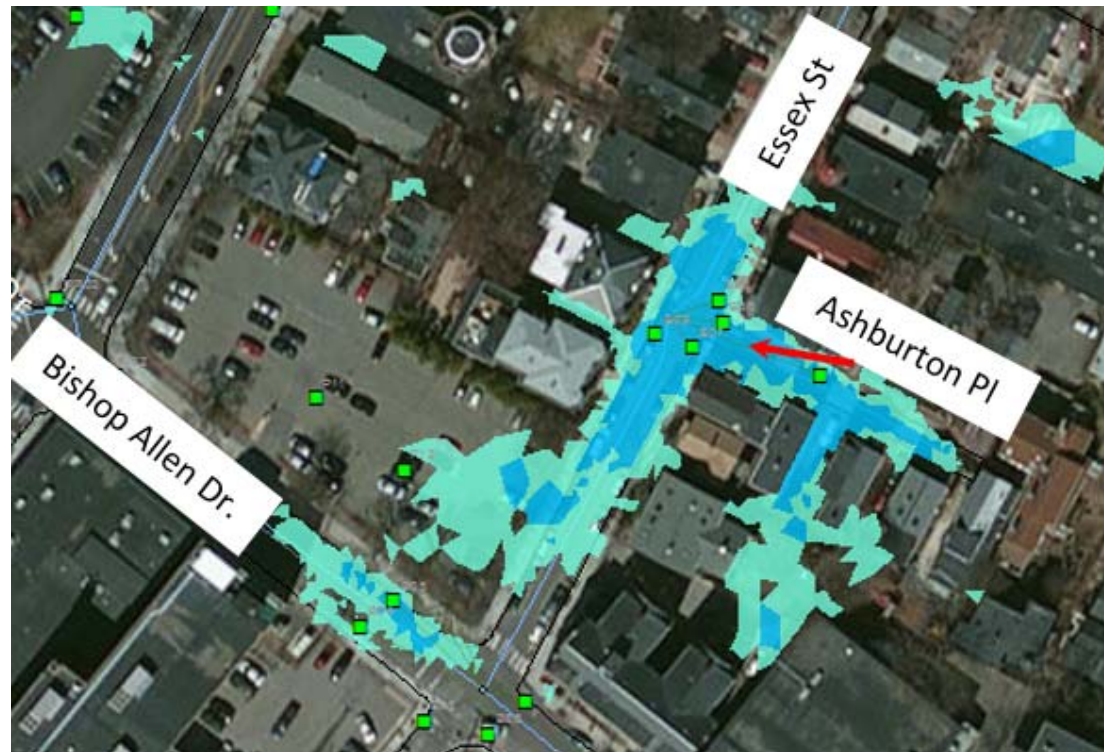
- LiDAR terrain data, impervious, and soil type GIS layers:
 - Used to assign Zone roughness coefficients for pervious/impervious areas, and infiltration parameters
- Building Footprint, and Road GIS Layers

Design Criteria

Design Solution Scenarios to Limit Flooding to within Right-of-Way.

- Reduce Damaging Flooding into Private Property

Design 'short-burst' Storm: 1.5 in/hour, 1-hour

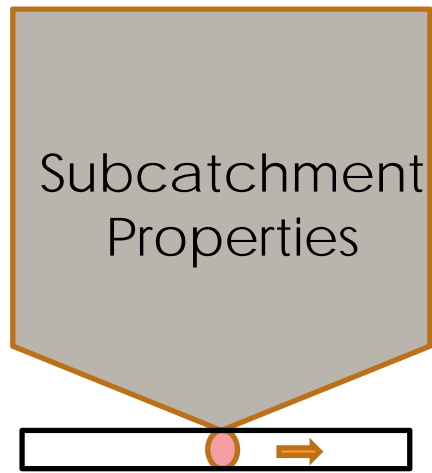


Numerical Model

- Existing Conditions Model
 - Citywide InfoWorks ICM Model
 - Underground sanitary and storm drain 1D model
 - Calibrated to Recent Metering Data
- Integrated 1D/2D Overland Flow Model
 - Covers ~70 Acres
 - Existing and Proposed Conditions
 - Uses HEC-22 (Urban Drainage Design Manual) Equations to model CB Hydraulics
 - Model Checks:
 - Against 1D model,
 - Mesh sensitivity

Numerical Model

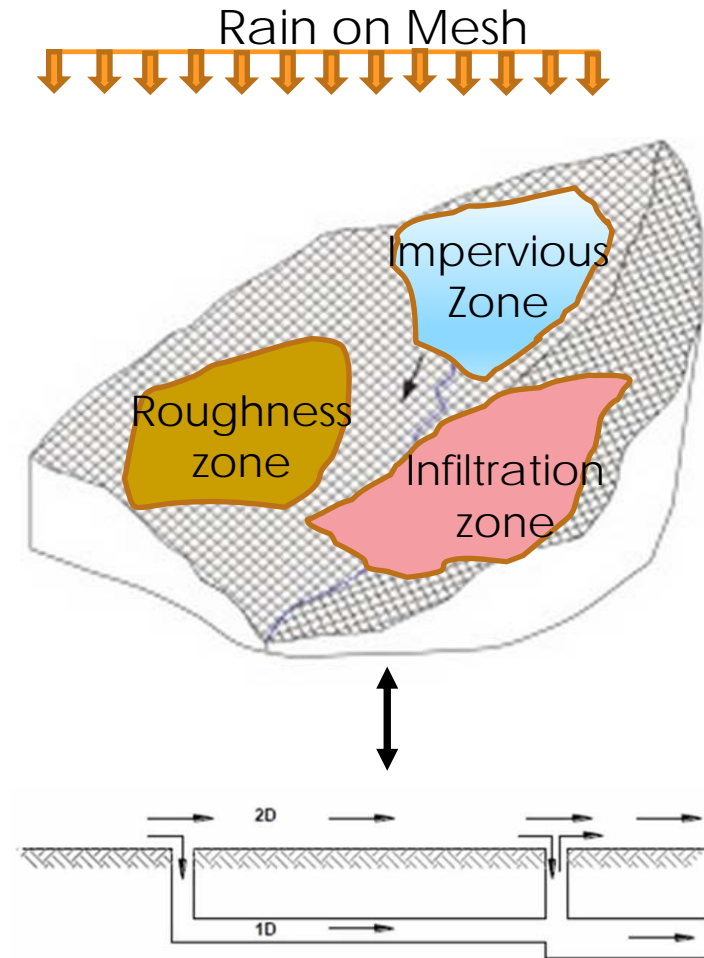
1D Model



Infiltration,
Roughness,
Imperviousness



1D/2D Overland Model



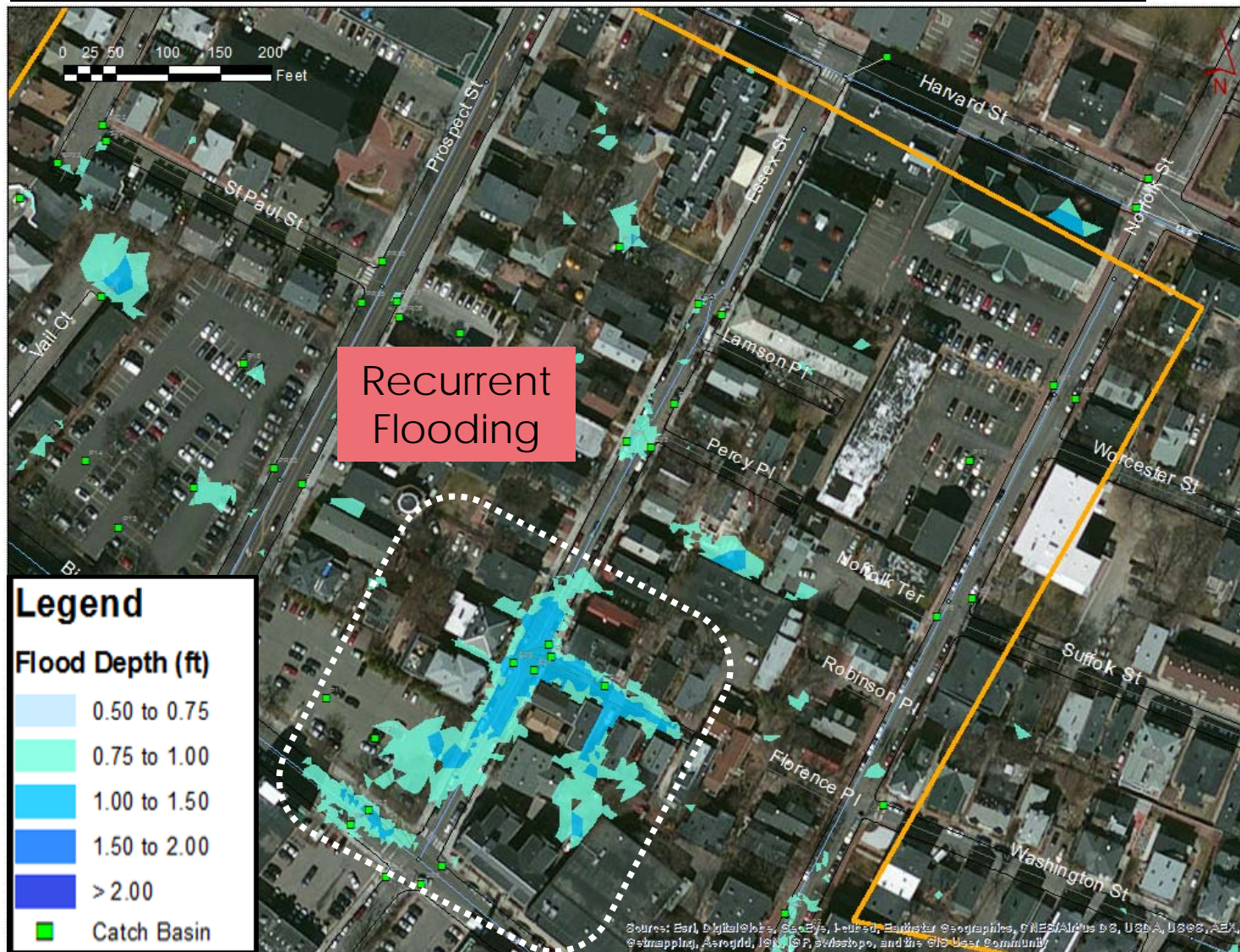
Numerical Model

Mesh sensitivity analysis



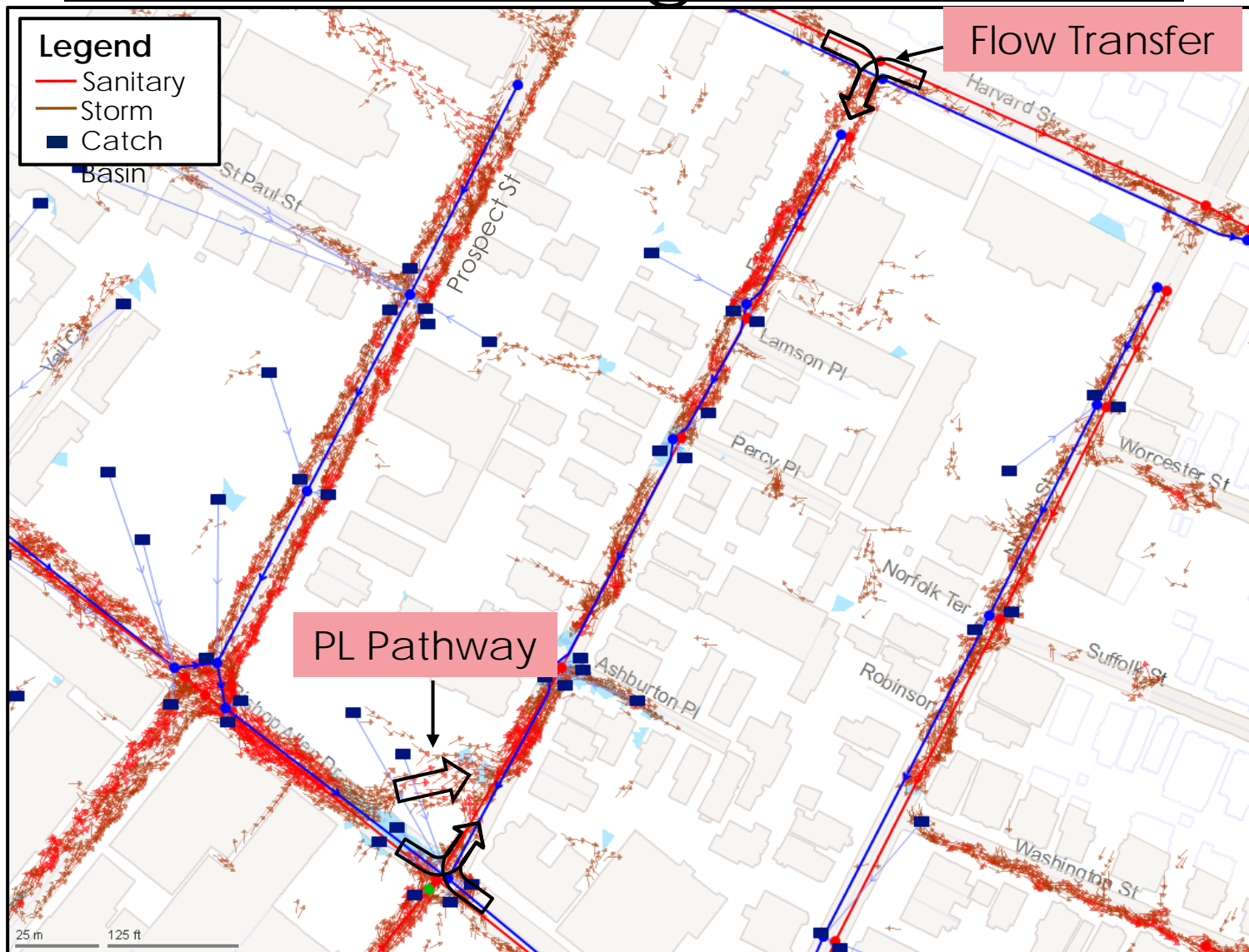
Evaluation of Existing Conditions

Results: Existing Conditions



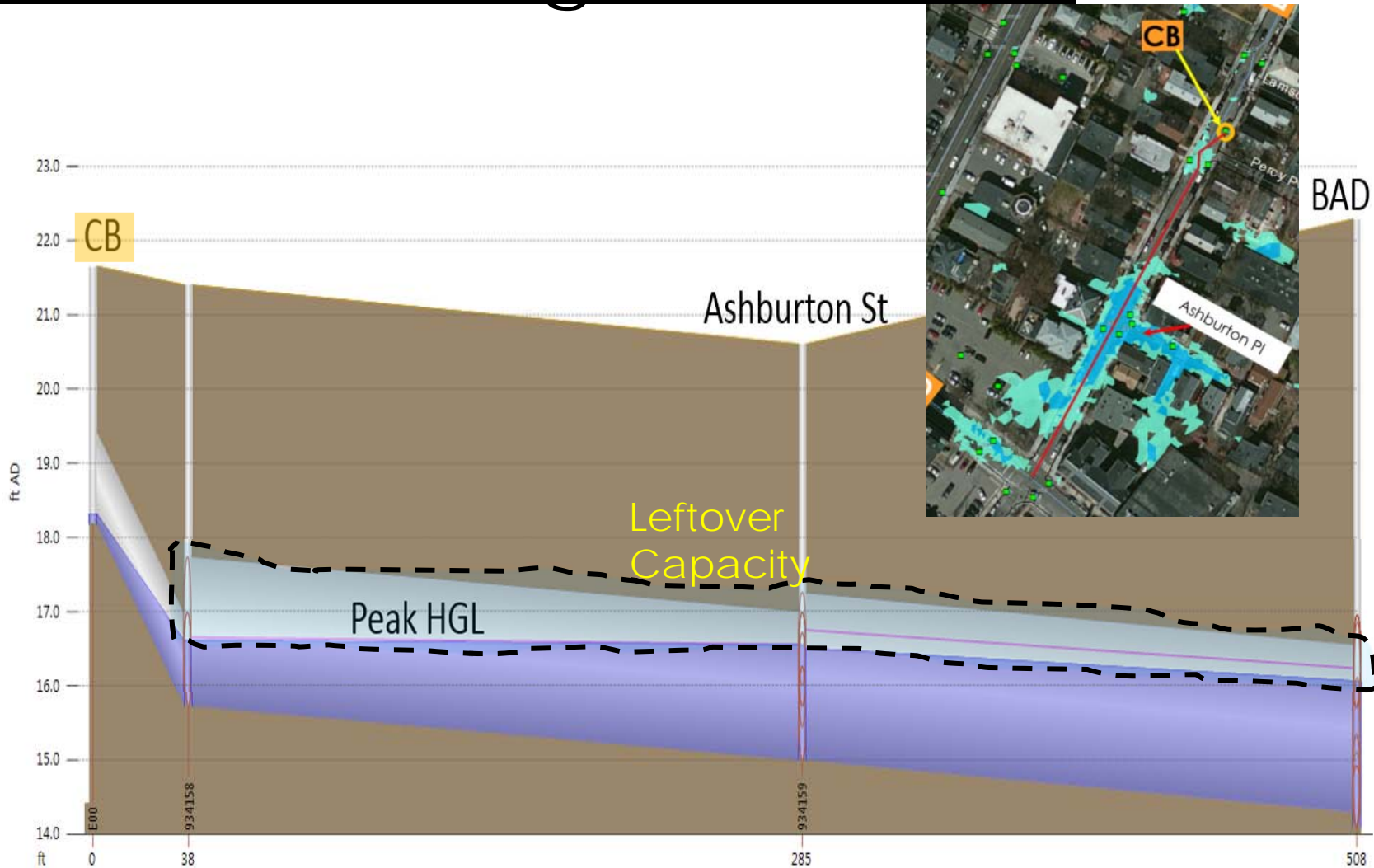
Simulated ground surface flooding during the 1.5 in/hour – 1 hour design storm under **Existing Conditions.**

Results: Existing Conditions



Simulated flow pathways into Ashburton Place during peak of the 1.5 in/hour – 1 hour design storm under **Existing Conditions**

Results: Existing Conditions



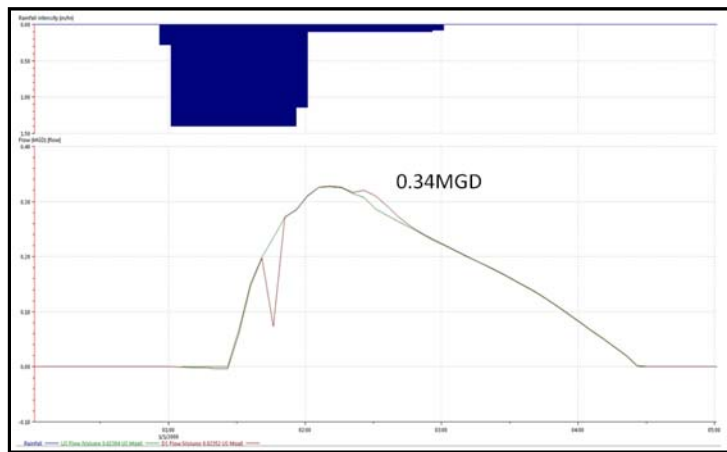
Simulated Peak HGL along Essex St. during the 1.5 in/hour – 1 hour design storm under existing conditions.

Evaluation of Flooding Solution Measures

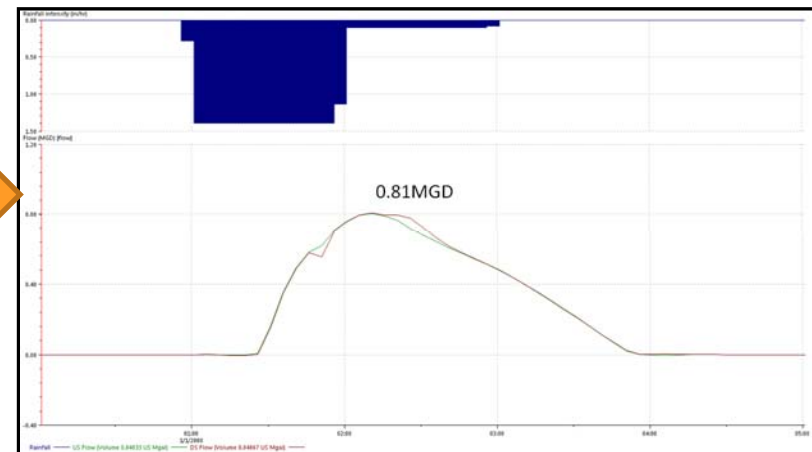
Results: Solution Scenario (Cont.)

Maximized CB Inflows

- New CBs were placed at lowest points



Added CB: Initial
Location



Added CB: Optimized
Location

Results: Solution Scenario (Cont.)



Pavement Resurfacing

Results: Solution Scenario (Cont.)



Simulated ground surface flooding during the 1.5 in/hour – 1 hour design storm under **Proposed Conditions**

Results: Solution Scenario (Cont.)



Simulated ground surface flooding during the 2.5 in/hour – 30 min design storm under **Proposed Conditions**

Summary & Conclusions

- Under Existing Conditions,
 - Street flooding is primarily attributed to limited inlet capacity,
 - The simulated peak flood depths in the area of interest rise well above 6-in,
 - Flood waters reach into driveways, building fronts, and backyards, while
 - The underground drain system has leftover capacity,

Summary & Conclusions (Cont.)

- Under Solution Scenario, several flooding control measures were evaluated:
 - Double-grating of existing inlets,
 - Adding single and double grate inlets,
 - Upsizing lead pipes,
 - Grating of Manhole
 - Raising of cross-walks and back of sidewalks, and
 - Pavement resurfacing

Summary & Conclusions (Cont.)

- Advantages of the 1D/2D Integrated Model
 - Allows Identification/Visualization of flow pathways into area of interest
 - Allows evaluation of multiple control measures either separately or in combination in a fast and cost effective manner
- Recommend Maintenance (e.g., clogging)



Thank you

Questions?

Additional

Sample CB Spread Analysis

Static Variables		Value	Units
Flow Analysis			
R = runoff yield	3.00		ft ³ / s * acre
Resultant Ponding			
n = Manning's coefficient	0.016		#
D _{MAX} = max allowable ponding depth	0.25		ft
T _{MAX} = max allowable ponding width	6.0		ft
Orifice Performance			
C _O = orifice constant	0.60		#
g = gravitational acceleration	32.20		ft ² / s

Site Conditions		Value	Units
H _{gutter} = gutter height			ft
L _{gutter} = gutter length			ft
L _{curb} = curb length			ft
H _{crown} = crown height			ft
L _{crown} = crown length			ft
L _{lane} = lane width			ft
A _T = total catch basin area			acres
S _L = longitudinal gutter slope			#
S _T = transverse gutter slope			#
A = total catch basin area			ft ²
Resultant Ponding			
D = depth of road ponding			ft
T = width of road ponding			ft

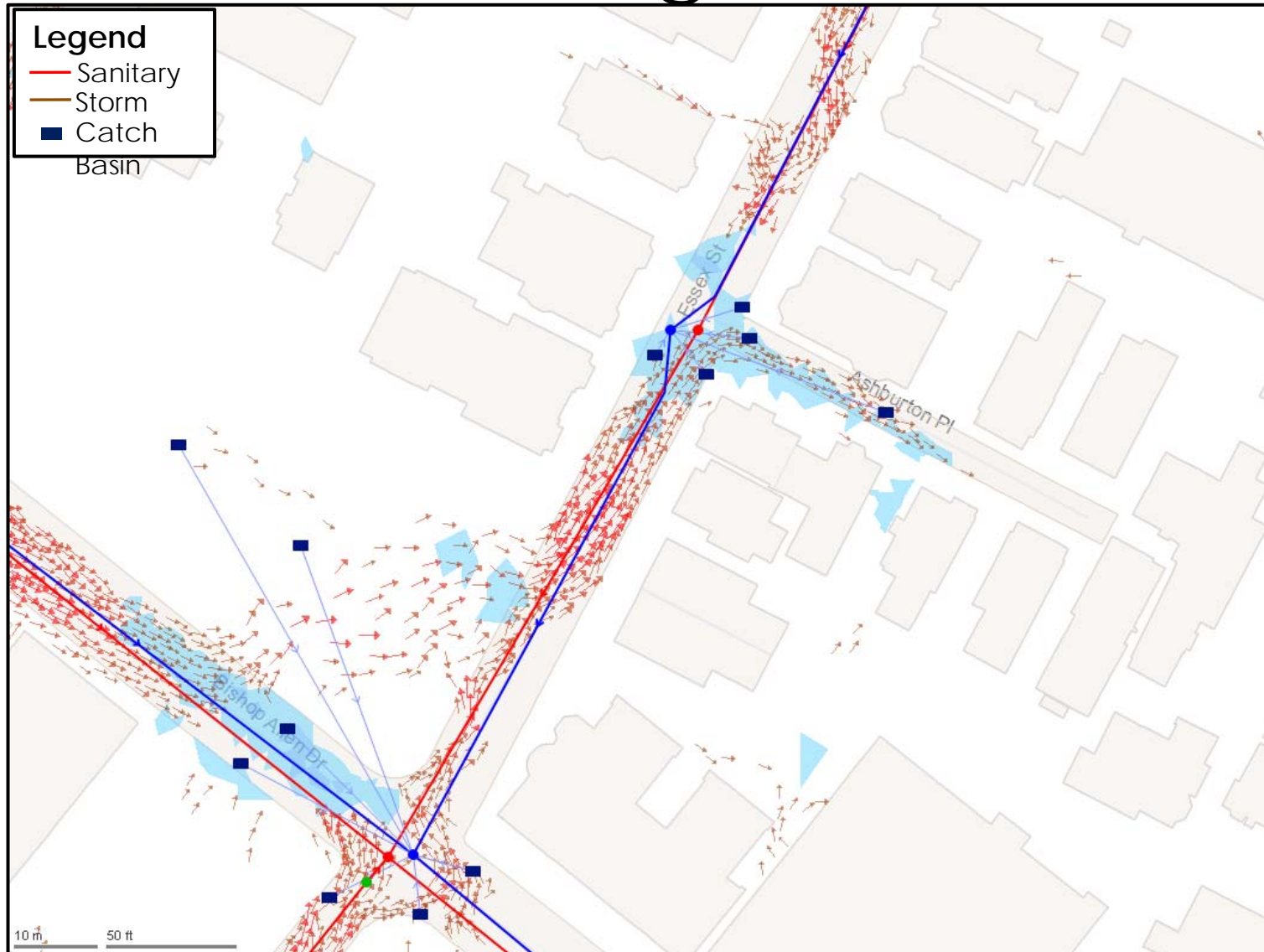
Flow Analysis		Value	Units
C = drainage coefficient			#
K _{GRATE} = grate minor loss coefficient			#
Q _{RAIN} = rain flow			ft ³ / s
Q _{TOTAL} = cumulative flow to CB			ft ³ / s
Q _{GRATE} = max capacity through CB			ft ³ / s
Q _{INT} = flow actually intercepted by CB			ft ³ / s
Q _{BYP} = flow bypassing CB			ft ³ / s
Q _{AVAIL} = remaining CB capacity			ft ³ / s
Orifice Performance			
G _A = open area of grate			ft ²
W = grate width			ft
X = averaged head acting on CB orifices			ft
Q _{ORIFICE} = cumulative flow through CB orifices			ft ³ / s

FORMULA BANK				ERROR	NEEHAH	"RECT. SLOT (TRADITIONAL)"	ERROR
This name convention does not apply any longer							

Contributing Area	CB Number	NEW CB	Street	Spread Flag	Manufacturer	Grate Type	A _T	Gutter		L _{curb}	Crown		L _{lane}
								H _{gutter}	L _{gutter}		H _{crown}	L _{crown}	
								New-Northwest1	CB-1		CB-1	New	
Larch-Northwest2	NA	CB-87	New	OK	NEEHAH	"RECT. SLOT (TRADITIONAL)"	0.0978	42.77	33.04	200	42.40	33.04	13
Larch-Northeast1	CB-2	CB-2	New	OK	NEEHAH	"RECT. SLOT (TRADITIONAL)"	0.202	53.34	42.16	200	42.40	42.16	13
Larch-Northeast2	NA	CB-88	New	FLOODED	NEEHAH	"RECT. SLOT (TRADITIONAL)"	1.0126	42.16	33.02	200	16.82	33.02	13
Larch-Sothwest1	CB-3	CB-3	New	OK	NEEHAH	"RECT. SLOT (TRADITIONAL)"	0.1002	52.81	46.61	100	46.87	46.61	13
Larch-Sothwest2	CB-4	CB-4	New	OK	NEEHAH	"RECT. SLOT (TRADITIONAL)"	0.1795	46.61	45.18	60	45.44	45.18	13
Larch-Sothwest3	CB-6	CB-6	New	FLOODED	NEEHAH	"RECT. SLOT (TRADITIONAL)"	2.1763	48.03	44.10	100	44.36	44.10	13
Larch-Sothwest4	CB-6.5	CB-8	New	FLOODED	NEEHAH	"RECT. SLOT (TRADITIONAL)"	0.3017	51.33	48.03	382	48.29	48.03	13
Larch-Sotheast1	CB-5	CB-5	New	OK	NEEHAH	"RECT. SLOT (TRADITIONAL)"	0.1167	52.42	44.87	150	45.13	44.87	13
Larch-Sotheast2	CB-7	CB-7	New	FLOODED	NEEHAH	"RECT. SLOT (TRADITIONAL)"	0.3127	51.17	44.47	506	44.73	44.47	13
Grozier-Northwest1	CB-8	CB-9	Grozier	OK	NEEHAH	"RECT. SLOT (TRADITIONAL)"	0.2243333	47.31	43.78	150	44.08	43.78	15
Grozier-Northeast1	CB-9	CB-10	Grozier	OK	NEEHAH	"RECT. SLOT (TRADITIONAL)"	0.1424	47.31	43.76	150	44.06	43.76	15
Grozier-Southwest1	CB-12	CB-11	Grozier	OK	NEEHAH	"RECT. SLOT (TRADITIONAL)"	0.1332417	48.62	48.56	40	48.86	48.56	15
Grozier-Southwest1	CB-12	CB-13	Grozier	OK	NEEHAH	"RECT. SLOT (TRADITIONAL)"	0.063225	48.62	48.56	40	48.86	48.56	15
Grozier-Southwest2	CB-13	CB-14	Grozier	OK	NEEHAH	"RECT. SLOT (TRADITIONAL)"	0.0784	48.56	47.88	75	48.18	47.88	15
Grozier-Southwest3	CB-15	CB-16	Grozier	FLOODED	NEEHAH	"RECT. SLOT (TRADITIONAL)"	0.4752	40.75	46.97	156	47.27	46.97	15

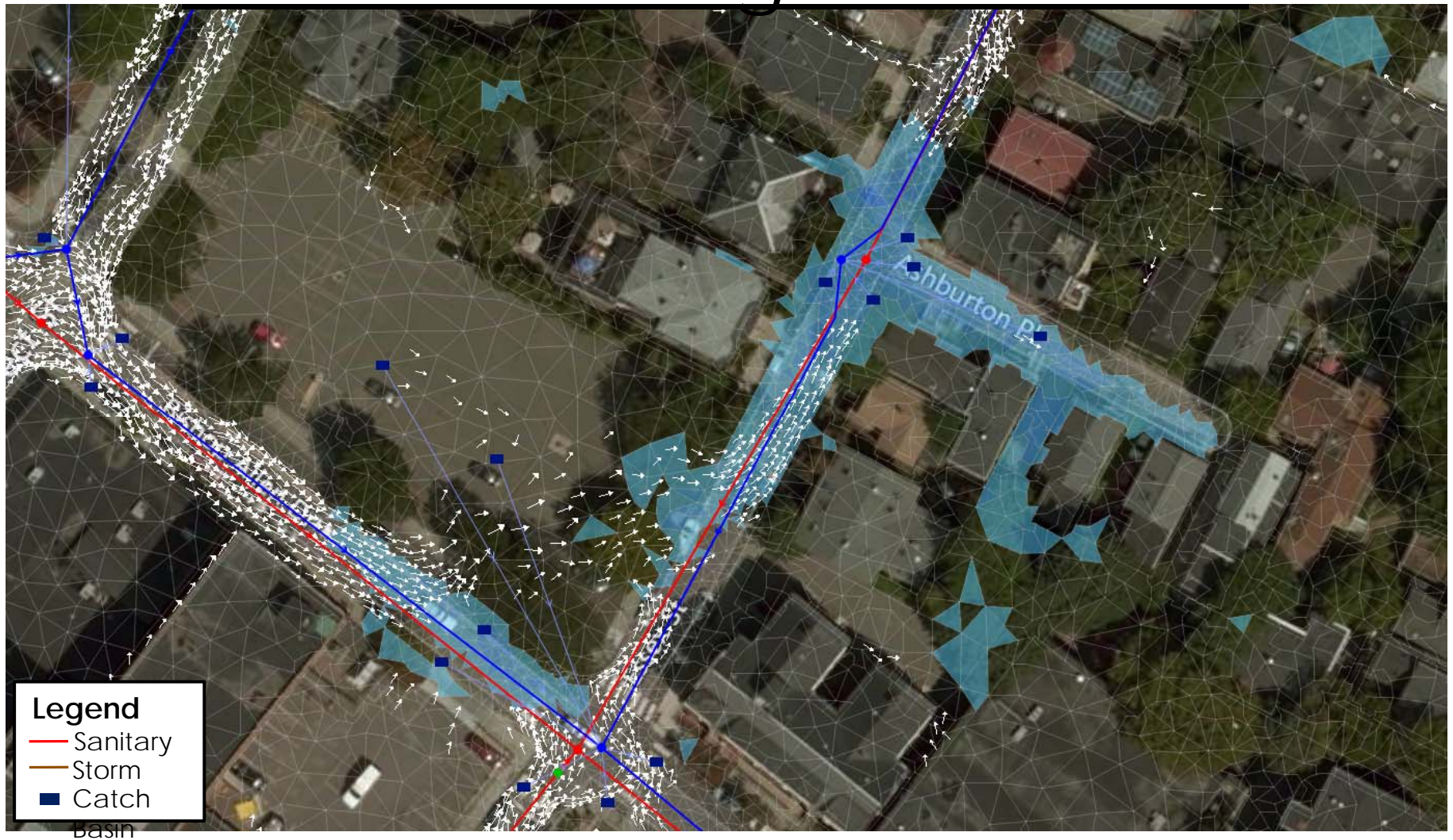


Results: Existing Conditions



Simulated flow pathways into Ashburton Place during peak of the 1.5 in/hour – 1 hour design storm under **Existing Conditions**

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